September 13, 2022

RB Engineering
91 SW 13th Street
Chehalis, WA 98532

Attention: Mr. Robert Balmelli, P.E.

Subject: Geotechnical Engineering Report
Wilson Commercial Development
2510 NE Kresky Avenue
Centralia, Washington
SSGC Project No. 22066

Mr. Balmelli,

South Sound Geotechnical Consulting (SSGC) has completed a geotechnical assessment for the planned Wilson Commercial development on the above addressed property in Centralia, Washington. Our services have been completed in general conformance with our proposal P22066 (dated July 11, 2022) and authorized per signature of our agreement for services. Our scope of services included completion of six test pits on the site, engineering analyses, and preparation of this report.

PROJECT INFORMATION

A new 12,000 square foot single-story commercial building is planned on the property northwest of the intersection of NE Kresky Avenue and NE Hampe Way. The site is currently undeveloped and used as parking.

SUBSURFACE CONDITIONS

Subsurface conditions were characterized by completing six test pits on the site on July 21, 2022. Explorations were advanced to depths between about 4 and 15 feet below existing ground surface. Approximate locations of the test holes are shown on Figure 1, Exploration Plan. A summary description of observed subgrade conditions is provided below. Logs of the test holes are provided in Appendix A.

Soil Conditions

Fill was observed in all of the test pits, with thicker fill in the western portion of the site. Fill consisted of three principal types:

1) Mixed silt, sand with variable gravel in a loose to medium dense condition in the northern and western portion of the site. This fill extended to depths between 2 and 6.5 feet.

2) Silty sand with gravel and occasional cobble below the above described fill in the western test pits. This fill was in a medium dense condition and extended to depths between 7 and 10.5 feet.

3) Clayey silt in the two test pits in the southeast portion of the site. This fill was in a soft condition and extended to depths between 1 to 4 feet.

Native soil below the fills was silt with clay and variable gravel to clayey silt. These soils were in a soft to stiff condition and extended to the termination depth of the test pits.

Groundwater Conditions

Groundwater was not observed in test pits at the time of excavation. Native soils are considered impermeable and can create perched groundwater, particularly during the wetter seasons of the year. Groundwater levels will vary throughout the year based on seasonal precipitation and on- and off-site drainage patterns.

Geologic Setting

Native soils are mapped as "Melbourne loam" per the USDA Soil Conservation Service of Lewis County. This soil reportedly formed in residuum from siltstone. Native soils in the test pits appear to conform to the mapped soil type.

GEOTECHNICAL DESIGN CONSIDERATIONS

Planned development of the site is considered feasible based on observed soil conditions in the test holes. However, construction over undocumented fill creates conditions that make structures susceptible to settlement. Typically, substantial over-excavation of fill and replacement with a zone of granular structural fill would be used for support of buildings using conventional spread footing foundations, or the use of deeper foundation elements (pile, piers). Considering the type of development planned for this site, lesser amounts of soil removal and replacement could be considered utilizing reinforced structural concrete mat-type foundations provided the owner is willing to accept the risk of possible limited future settlement.

Recommendations presented in the following sections should be considered general and may require modifications when earthwork and grading occur. They consider that reinforced mat foundations will be used for support of the storage buildings. They are based upon the subsurface conditions observed in the test pits and the assumption that finish site grades will be similar to existing grades, or slightly higher. It should be noted that subsurface conditions across the site may vary from those depicted on the exploration logs and can change with time. Therefore, proper site preparation will depend upon the weather and soil conditions encountered at the time of construction. We recommend that SSGC review final plans and further assess subgrade conditions at the time of construction, as warranted.

General Site Preparation

Site grading and earthwork should include procedures to control surface water runoff. Grading the site without adequate drainage control measures may negatively impact site soils, resulting in increased export of impacted soil and import of fill materials, potentially increasing the cost of the earthwork and subgrade preparation phases of the project.

Site grading should include removal (stripping) of topsoil and debris rich fill in building and pavement areas. Subgrades should consist of firm debris-less soils following stripping. Stripping depth will vary across the site. Final stripping depths can only be determined at the time of construction.

General Subgrade Preparation

Subgrades in building and pavement areas should consist of firm soil. We recommend exposed subgrades in building and pavement areas are proofrolled using a large roller, loaded dump truck, or other mechanical equipment to assess subgrade conditions following stripping. Proofrolling efforts should result in the upper 1 foot of subgrade soils achieving a firm and unyielding condition and a compaction level of at least 92 percent of the maximum dry density (MDD) per the ASTM D1557 test method. Wet, loose, or soft subgrades that cannot achieve a firm and unyielding condition should be removed (over-excavated) and replaced with structural fill. The depth of over-excavation should be based on soil conditions at the time of construction. A representative of SSGC should be present to assess subgrade conditions during proofrolling.

Grading and Drainage

Positive drainage should be provided during construction and maintained throughout the life of the development. Allowing surface water into cut or fill areas, utility trenches, and building footprints should be prevented.

Structural Fill Materials

The suitability of soil for use as structural fill will depend on the gradation and moisture content of the soil when it is placed. Soils with higher fines content (soil fraction passing the U.S. No. 200 sieve) will become sensitive with higher moisture content. It is often difficult to achieve adequate compaction if soil moisture is outside of optimum ranges for soils that contain more than about 5 percent fines.

> Site Soils: Granular fill observed in the test pits could potentially be used for structural fill, but should be evaluated at the time of construction. Native soils are fine grained (principally silt and clay) making them are moisture sensitive and would be difficult to use as structural fill unless conditioned to optimum moisture content. Optimum moisture is considered within about +/- 2 percent of the moisture content required to achieve the maximum density per the ASTM D-1557

test method. If moisture content is higher or lower than optimum, soils would need to be dried or wetted prior to placement as structural fill.

Import Fill Materials: We recommend import structural fill placed during dry weather periods consist of material which meets the specifications for *Gravel Borrow* as described in Section 9-03.14(1) of the Washington State Department of Transportation (WSDOT) Specifications for Road, Bridge, and Municipal Construction (Publication M41-10). Gravel Borrow should be protected from disturbance if exposed to wet conditions after placement.

During wet weather, or for backfill on wet subgrades, import soil suitable for compaction in wetter conditions should be provided. Imported fill for use in wet conditions should generally conform to specifications for *Select Borrow* as described in Section 9-03.14(2), or *Crushed Surfacing* per Section 9-03.9(3) of the WSDOT M41-10 manual, with the modification that a maximum of 5 percent by weight shall pass the U.S. No. 200 sieve.

It should be noted that structural fill placement and compaction is weather-dependent. Delays due to inclement weather are common, even when using select granular fill. We recommend site grading and earthwork be scheduled for the drier months of the year. Structural fill should not consist of frozen material.

Structural Fill Placement

We recommend structural fill is placed in lifts not exceeding about 10 inches in loose measure. It may be necessary to adjust lift thickness based on site and fill conditions during placement and compaction. Finer grained soil used as structural fill and/or lighter weight compaction equipment may require significantly thinner lifts to attain required compaction levels. Granular soil with lower fines contents could potentially be placed in thicker lifts if they can be adequately compacted. Structural fill should be compacted to attain the recommended levels presented in Table 1, Compaction Criteria.

Table 1. Compaction Criteria

Fill Application	Compaction Criteria*
Footing areas (below structures and retaining walls)	95 %
Upper 2 feet in pavement areas, slabs and sidewalks, and utility trenches	95 %
Below 2 feet in pavement areas, slabs and sidewalks, and utility trenches	92 %
Utility trenches or general fill in non-paved or -building areas	90 %

*Per the ASTM D 1557 test method.

Trench backfill within about 2 feet of utility lines should not be over-compacted to reduce the risk of damage to the line. In some instances, the top of the utility line may be within 2 feet of the surface. Backfill in these circumstances should be compacted to a firm and unyielding condition.

We recommend fill procedures include maintaining grades that promote drainage and do not allow ponding of water within the fill area. The contractor should protect compacted fill subgrades from disturbance during wet weather. In the event of rain during structural fill placement, the exposed fill surface should be allowed to dry prior to placement of additional fill. Alternatively, the wet soil can be removed. We recommend consideration be given to protecting haul routes and other high traffic areas with free-draining granular fill material (i.e. sand and gravel containing less than 5 percent fines) or quarry spalls to reduce the potential for disturbance to the subgrade during inclement weather.

We recommend a separation fabric (such as Mirafi 140N) is placed on prepared subgrades prior to placement of the structural fill section. The purpose of the fabric is to provide segregation between new granular structural fill and the finer grained native (or soft fill) soil. Without the fabric, the new structural fill will have the tendency to migrate into the softer/looser subgrade soil over time, which can compromise the structural integrity of the structural fill zone leading to premature distress of the fill section.

Earthwork Procedures

Conventional earthmoving equipment should be suitable for earthwork at this site. Earthwork may be difficult during periods of wet weather or if elevated soil moisture is present. Excavated site soils may not be suitable as structural fill depending on the soil moisture content and weather conditions at the time of earthwork. If soils are stockpiled and wet weather is anticipated, the stockpile should be protected with securely anchored plastic sheeting. If stockpiled soils become unusable, it may become necessary to import clean, granular soils to complete wet weather site work.

Wet or disturbed subgrade soils should be over-excavated to expose firm, non-yielding, non-organic soils and backfilled with compacted structural fill. We recommend the earthwork portion of this project be completed during extended periods of dry weather. If earthwork is completed during the wet season (typically late October through May) it may be necessary to take extra measures to protect subgrade soils.

If earthwork takes place during freezing conditions, we recommend exposed subgrades are allowed to thaw and re-compacted prior to placing subsequent lifts of structural fill. Alternatively, the frozen soil can be removed to unfrozen soil and replaced with structural fill.

The contractor is responsible for designing and constructing stable, temporary excavations (including utility trenches) as required to maintain stability of excavation sides and bottoms. Excavations should be sloped or shored in the interest of safety following local and federal regulations, including current OSHA excavation and trench safety standards. Temporary excavation cuts should be sloped at inclinations of 1H:1.5V (Horizontal:Vertical) or flatter, unless the contractor can demonstrate the safety of steeper

inclinations. Shoring may be required in deeper excavations (below 4 feet) as soft soils may cave into open excavations.

A geotechnical engineer and accredited material testing laboratory should be retained during the construction phase of the project to observe earthwork operations and perform necessary tests and observations during subgrade preparation, placement and compaction of structural fill, and backfilling of excavations.

Foundations

We recommend foundations are placed on a zone of granular structural fill above prepared subgrade soils prepared as described in this report. This zone should be a minimum of two feet thick (including existing fill thickness) and compacted to a firm and unyielding condition. The purpose of the structural fill pad and mat foundation is to limit differential settlement across the footprint of the building. We recommend a separation fabric (such as Mirafi 140N or equivalent) is placed on the prepared subgrade prior to fill placement. The following recommendations are for reinforced mat foundations:

Bearing Capacity (net allowable):	1,500 pounds per square foot (psf) for reinforced mat foundations supported on a zone of compacted granular fill over native soils prepared as described in this report.
Footing Width (Minimum):	18 inches (Strip) 24 inches (Column)
Embedment Depth (Minimum):	18 inches (Exterior) 12 inches (Interior)
Settlement:	Total: < 1.5 inch Differential: < 1/2 inch (over 30 feet)
Allowable Lateral Passive Resistance:	300 psf/ft* (below 18 inches)
Allowable Coefficient of Friction:	0.35*

*These values include a factor of safety of approximately 1.5.

The net allowable bearing pressures presented above may be increased by one-third to resist transient, dynamic loads such as wind or seismic forces. Lateral resistance to footings should be ignored in the upper 12-inches from exterior finish grade. Although conventional spread footings could be used for support of the building, the owner should be aware that additional differential settlement could occur.

Foundation Construction Considerations

All foundation subgrades should be free of water and loose soil prior to placing concrete and should be prepared as recommended in this report. Concrete should be placed soon after excavating and compaction to reduce disturbance to bearing soils. Should soils at foundation level become excessively dry, disturbed, saturated, or frozen, the affected soil should be removed prior to placing concrete. We recommend SSGC observe all foundation subgrades prior to placement of concrete.

Foundation Drainage

Ground surface adjacent foundations should be sloped away from the building. We recommend footing drains are installed around perimeter thickened edge footings. Footing drains should include a minimum 4-inch diameter perforated rigid plastic drain line installed at the base of the footing. The perforated drain lines should be connected to a tight line pipe that discharges to an approved storm drain receptor. The drain line should be surrounded by a zone of clean, free-draining granular material having less than 5 percent passing the No. 200 sieve or meeting the requirements of section 9-03.12(2) "Gravel Backfill for Walls" in the 2018 WSDOT Standard Specifications for Road, Bridge, and Municipal Construction manual (M41-10). The free-draining aggregate zone should be at least 12 inches wide and wrapped in filter fabric. The granular fill should extend to within 6 inches of final grade where it should be capped with compacted fill containing sufficient fines to reduce infiltration of surface water into the footing drains. Cleanouts are recommended for maintenance of the drain system.

On-Grade Floor Slabs

On-grade floor slabs should be placed on prepared subgrades as described in this report. We recommend a modulus subgrade reaction of 125 pounds per square inch per inch (psi/in) for floor slabs on subgrade soils.

We recommend a capillary break is provided between the prepared subgrade and bottom of slab. Capillary break material should be a minimum of 4 inches thick and consist of compacted clean, free-draining, well graded coarse sand and gravel. The capillary break material should contain less than 5 percent fines, based on that soil fraction passing the U.S. No. 4 sieve. Alternatively, a clean angular gravel such as No. 7 aggregate per Section 9-03.1(4)C of the 2018 WSDOT (M41-10) manual could be used for this purpose.

Seismic Considerations

Seismic parameters and values in Table 2 are recommended based on the 2018 International Building Code (IBC).

Table 2. Seismic Parameters

PARAMETER	VALUE
2018 International Building Code (IBC) Site Classification[1]	E
S_s Spectral Acceleration for a Short Period	1.195
S_1 Spectral Acceleration for a 1-Second Period	0.487g

[1] Note: In general accordance with the *2018 International Building Code*, for risk categories I,II,III. IBC Site Class is based on the estimated characteristics of the upper 100 feet of the subsurface profile.

Liquefaction

Soil liquefaction is a condition where loose, typically granular soils located below the groundwater surface lose strength during ground shaking, and is often associated with earthquakes. The risk of liquefaction at this site is considered low due to the overall fine-grained nature of native site soils. However, some deformation of underlying soft soils at this site should be expected. Although structural failure of foundations is not anticipated, some limited structural damage could occur during a seismic event.

Conventional Asphalt Pavement Sections

Subgrades for conventional pavement areas should be prepared as described in the "*Subgrade Preparation*" section of this report. Subgrades below pavement sections should be graded or crowned to promote drainage and not allow for ponding of water beneath the section. If drainage is not provided and ponding occurs, subgrade soils could become saturated, lose strength, and result in premature distress or failure of the section. In addition, the pavement surfacing should also be graded to promote drainage and reduce the potential for ponding of water on the pavement surface.

We recommend a separation fabric (such as Mirafi 140N) is placed on new pavement subgrades that consist of native clayey silt prior to placement of structural or pavement section fill. The purpose of the fabric is to provide segregation between new granular structural fill and the finer grained subgrade soil. Without the fabric, the new granular fill will have the tendency to migrate into the softer/looser subgrade soil over time, which can compromise the structural integrity of the structural fill zone leading to premature distress of the pavement section. A separation fabric is not considered necessary for pavement subgrades that consist of existing granular fill.

Minimum recommended pavement sections for conventional asphalt pavements are presented in Table 3. New pavement sections in public right-of-ways should conform to City of Centralia standards.

Table 3. Minimum Pavement Sections

Traffic Area	Minimum Recommended Pavement Section Thickness (inches)			
	Asphalt Concrete Surface[1]	Portland Cement Concrete[2]	Aggregate Base Course[3,4]	Subbase Aggregate[5]
Heavy Traffic	3	6	4	12
Parking Areas	2	5	4	12

[1] 1/2 –inch nominal aggregate hot-mix asphalt (HMA) per WSDOT 9-03.8(1)

[2] A 28-day minimum compressive strength of 4,000 psi and an allowable flexural strength of at least 250 psi

[3] Crushed Surfacing Base Course per WSDOT 9-03.9(3)

[4]Although not required for structural support under concrete pavements, a minimum four-inch-thick base course layer is recommended to help reduce potential for slab curl, shrinkage cracking, and subgrade "pumping" through joints

[5] 95% compacted native subgrade or Gravel Borrow per WSDOT 9-03.14(1) or Crushed Surfacing Base Course WSDOT 9-03.9(3)

Conventional Pavement Maintenance

The performance and lifespan of pavements can be significantly impacted by future maintenance. The above pavement sections represent minimum recommended thicknesses and, as such, periodic maintenance should be completed. Proper maintenance will slow the rate of pavement deterioration and will improve pavement performance and life. Preventive maintenance consists of both localized maintenance (crack and joint sealing and patching) and global maintenance (surface sealing). Added maintenance measures should be anticipated over the lifetime of the pavement section if any fill or topsoil is left in-place beneath pavement sections.

REPORT CONDITIONS

This report has been prepared for the exclusive use of RB Engineering for specific application to the project discussed and has been prepared in accordance with generally accepted geotechnical engineering practices in the area. No warranties, either express or implied, are intended or made. The analysis and recommendations presented in this report are based on observed soil conditions and test results at the indicated locations, and from other geologic information discussed. This report does not reflect variations that may occur across the site, or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. If variations appear, we should be immediately notified so that further evaluation and supplemental recommendations can be provided.

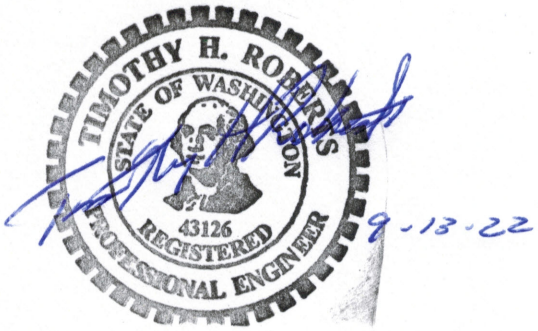
This report was prepared for the planned type of development of the site as discussed herein. It is not valid for third party entities or alternate types of development on the site without the express written consent of SSGC. If development plans change, we should be notified to review those changes and modify our recommendations as necessary.

The scope of services for this project does not include any environmental or biological assessment of the site including identification or prevention of pollutants, hazardous materials, or conditions. Other studies should be completed if the owner is concerned about the potential for contamination or pollution.

We appreciate the opportunity to work with you on this project. Please contact us if additional information is required or we can be of further assistance.

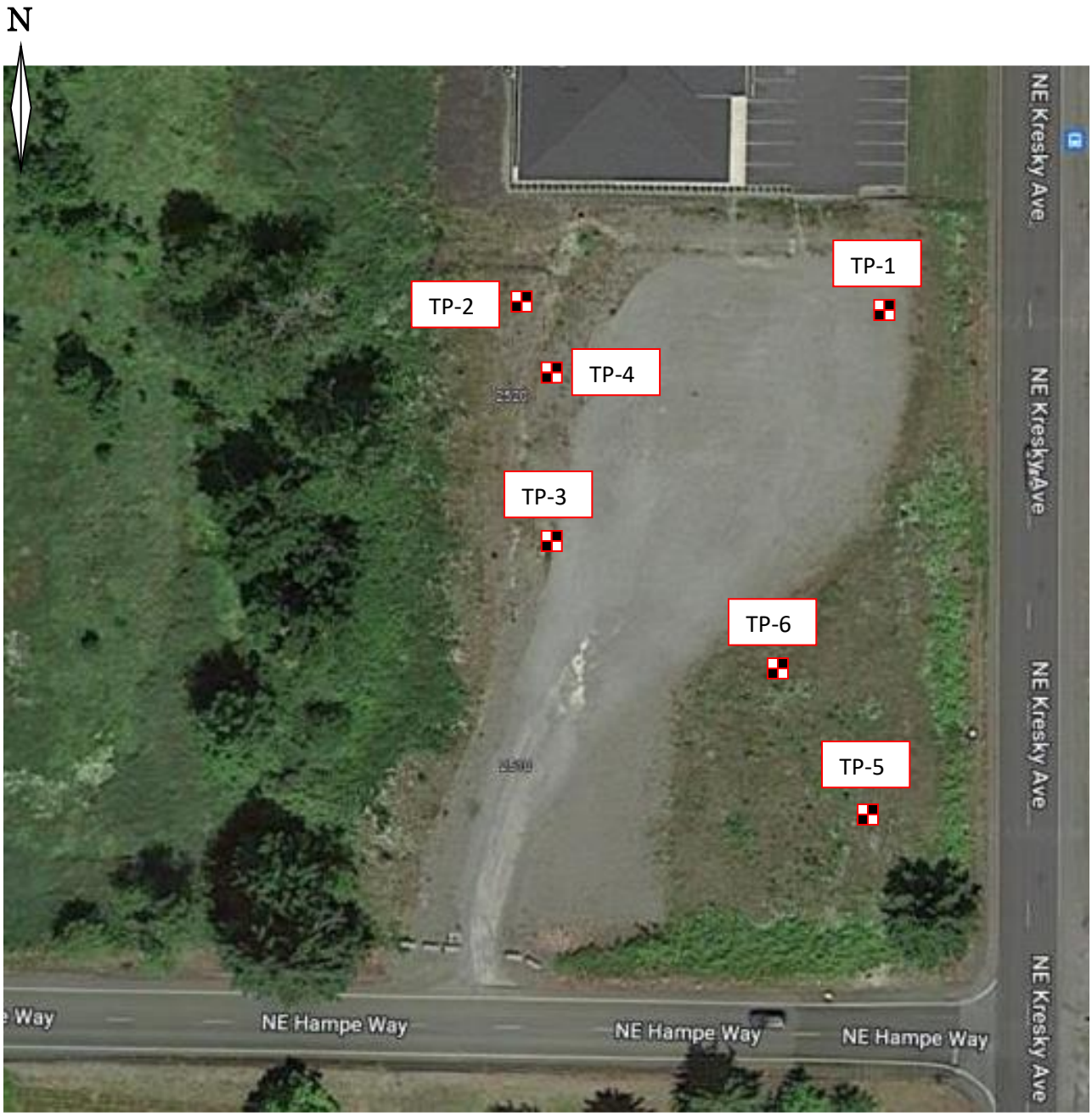
Respectfully,

South Sound Geotechnical Consulting



Timothy H. Roberts, P.E.
Member/Geotechnical Engineer

Attachments: Figure 1 – Exploration Plan
Appendix A – Field Exploration Procedures and Test Pit Logs
Unified Soil Classification System



Legend

TP - 1

Approximate Test Pit Location

Base map from Google Maps.

Scale: NTS

South Sound Geotechnical Consulting
P.O. Box 39500
Lakewood, WA 98496
(253) 973-0515

Figure 1 – Exploration Plan

Wilson Commercial
Centralia, Washington

SSGC Project #22066

Appendix A

Field Exploration Procedures and Test Pit Logs

Field Exploration Procedures

Our field exploration for this project included six test pits completed on July 21, 2022. The approximate locations of the explorations are shown on Figure 1, Exploration Plan. Exploration locations were determined by pacing from site features. Test pit locations should be considered accurate only to the degree implied by the means and methods used.

A private excavation contractor dug the test pits. Select soil samples were collected and stored in moisture tight containers for further assessment and laboratory testing. Explorations were backfilled with excavated soils and tamped when completed. Please note that backfill in the explorations may settle with time. Backfill material located in building or pavement areas should be re-excavated and recompacted, or replaced with structural fill.

The following logs indicate the observed lithology of soils and other materials observed in the explorations at the time of excavation. Where a soil contact was observed to be gradational, our log indicates the average contact depth. Our logs also indicate the approximate depth to groundwater (where observed at the time of excavation), along with sample numbers and approximate sample depths. Soil descriptions on the logs are based on the Unified Soil Classification System.

Test Pit TP-1

Depth (feet)	Material Description
0 – 2	**Fill:** Surface gravel over mixed silt, sand, with occasional gravel: Loose to medium dense, damp, brown.
2 – 8	SILT with clay, sand, and gravel: Stiff, moist, rust brown. (ML)
	Test pit completed at approximately 5 feet on 7/21/22. Groundwater not observed at feet at time of excavation.

Test Pit TP-2

Depth (feet)	Material Description
0 – 6.5	**Fill:** Mixed silt, sand, with occasional gravel: Loose to medium dense, damp, brown.
6.5 – 10.5	**Fill:** Silty sand with gravel and cobbles: Medium dense, moist, dark brown to black.
10.5 – 15	Clayey SILT: Soft to stiff, wet, grayish green with orange mottling.(ML)
	Test pit completed at approximately 15 feet on 7/21/22. Groundwater not observed at time of excavation.

Test Pit TP-3

Depth (feet)	Material Description
0 – 3.5	**Fill:** Mixed silt, sand, with occasional gravel: Loose to medium dense, damp, brown.
3.5 – 9	**Fill:** Silty sand with gravel and cobbles: Medium dense, moist, dark brown to black.
9 – 10.5	Clayey SILT with roots: Soft, wet, dark gray.(ML)
10.5 – 12	Clayey SILT: Soft to stiff, wet, grayish green with orange mottling. (ML)
	Test pit completed at approximately 12 feet on 7/21/22. Groundwater not observed at time of excavation.

Test Pit TP-4

Depth (feet)	Material Description
0 – 4	**Fill:** Mixed silt, sand, with occasional gravel: Loose to medium dense, damp, brown.
4 – 7	**Fill:** Silty sand with gravel and cobbles: Medium dense, moist, dark brown to black.
7 – 8.5	Clayey SILT with roots: Soft, wet, dark gray. (ML)
8.5 – 10	Clayey SILT: Soft to stiff, wet, grayish green with orange mottling**.** (ML)
	Test pit completed at approximately 10 feet on 7/21/22. Groundwater not observed at time of excavation.

Test Pit TP-5

Depth (feet)	Material Description
0 – 4	**Fill:** Clayey SILT: Soft, moist, brown to gray.
4 – 6	SILT with clay, sand, and gravel: Stiff, moist, gray with orange mottling. (ML)
	Test pit completed at approximately 6 feet on 7/21/22. Groundwater not observed at time of excavation.

Test Pit TP-6

Depth (feet)	Material Description
0 – 1	**Fill:** Clayey SILT: Soft, moist, brown to gray.
1 – 4	SILT with clay, sand, and gravel: Stiff, moist, gray with orange mottling. (ML)
	Test pit completed at approximately 4 feet on 7/21/22. Groundwater not observed at time of excavation.

UNIFIED SOIL CLASSIFICATION SYSTEM

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests[A]					Soil Classification	
					Group Symbol	Group Name[B]
Coarse Grained Soils More than 50% retained on No. 200 sieve	Gravels More than 50% of coarse fraction retained on No. 4 sieve	Clean Gravels Less than 5% fines[C]	$Cu \geq 4$ and $1 \leq Cc \leq 3$[E]		GW	Well-graded gravel[F]
			$Cu < 4$ and/or $1 > Cc > 3$[E]		GP	Poorly graded gravel[F]
		Gravels with Fines More than 12% fines[C]	Fines classify as ML or MH		GM	Silty gravel[F,G,H]
			Fines classify as CL or CH		GC	Clayey gravel[F,G,H]
	Sands 50% or more of coarse fraction passes No. 4 sieve	Clean Sands Less than 5% fines[D]	$Cu \geq 6$ and $1 \leq Cc \leq 3$[E]		SW	Well-graded sand[I]
			$Cu < 6$ and/or $1 > Cc > 3$[E]		SP	Poorly graded sand[I]
		Sands with Fines More than 12% fines[D]	Fines classify as ML or MH		SM	Silty sand[G,H,I]
			Fines Classify as CL or CH		SC	Clayey sand[G,H,I]
Fine-Grained Soils 50% or more passes the No. 200 sieve	Silts and Clays Liquid limit less than 50	inorganic	PI > 7 and plots on or above "A" line[J]		CL	Lean clay[K,L,M]
			PI < 4 or plots below "A" line[J]		ML	Silt[K,L,M]
		organic	Liquid limit - oven dried	< 0.75	OL	Organic clay[K,L,M,N]
			Liquid limit - not dried			Organic silt[K,L,M,O]
	Silts and Clays Liquid limit 50 or more	inorganic	PI plots on or above "A" line		CH	Fat clay[K,L,M]
			PI plots below "A" line		MH	Elastic Silt[K,L,M]
		organic	Liquid limit - oven dried	< 0.75	OH	Organic clay[K,L,M,P]
			Liquid limit - not dried			Organic silt[K,L,M,Q]
Highly organic soils	Primarily organic matter, dark in color, and organic odor				PT	Peat

[A] Based on the material passing the 3-in. (75-mm) sieve

[B] If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.

[C] Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.

[D] Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay

[E] $Cu = D_{60}/D_{10}$ $\quad Cc = \dfrac{(D_{30})^2}{D_{10} \times D_{60}}$

[F] If soil contains ≥ 15% sand, add "with sand" to group name.

[G] If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

[H] If fines are organic, add "with organic fines" to group name.

[I] If soil contains ≥ 15% gravel, add "with gravel" to group name.

[J] If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.

[K] If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.

[L] If soil contains ≥ 30% plus No. 200 predominantly sand, add "sandy" to group name.

[M] If soil contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name.

[N] PI ≥ 4 and plots on or above "A" line.

[O] PI < 4 or plots below "A" line.

[P] PI plots on or above "A" line.

[Q] PI plots below "A" line.